

**Warsaw University  
of Technology**



**Faculty of Power and  
Aeronautical Engineering**

WARSAW UNIVERSITY OF TECHNOLOGY

Institute of Aeronautics and Applied Mechanics

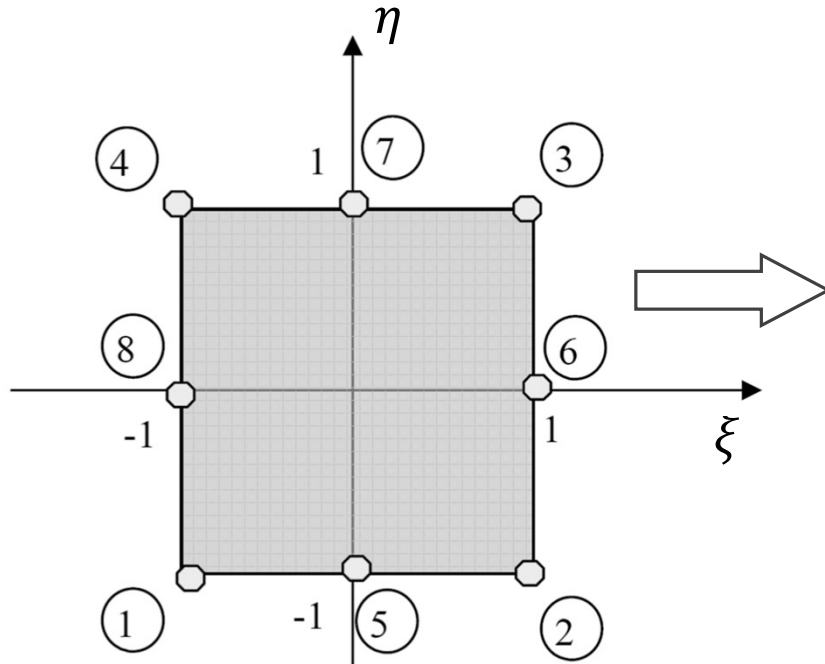
# Finite element method (FEM)

8-node quadrilateral element

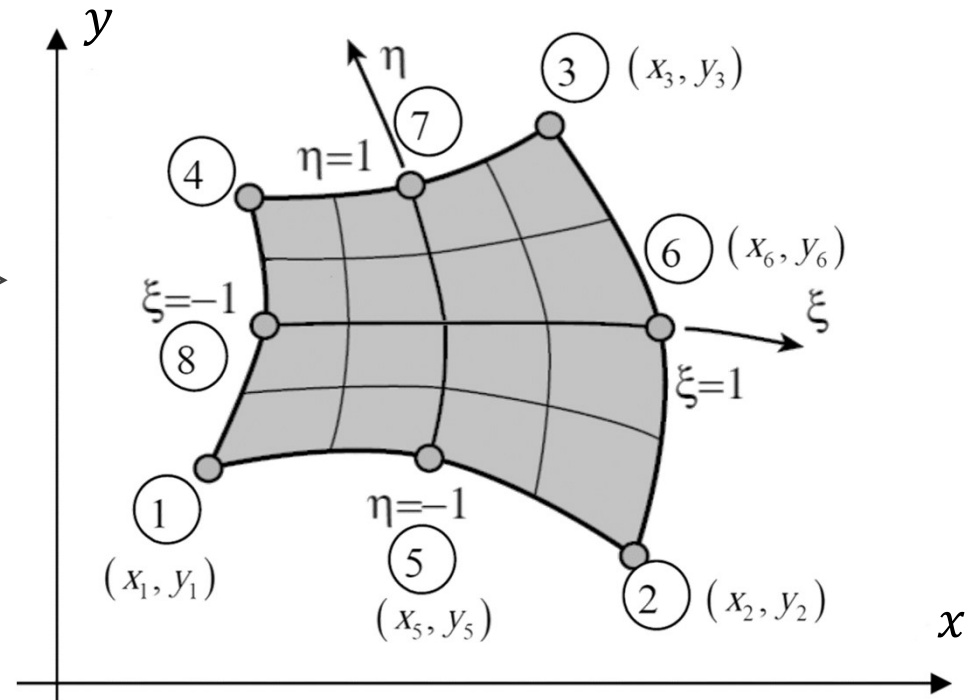
03.2021

# 8-node 2D quadrilateral element (accuracy, irregular shapes)

natural coordinate system



cartesian coordinate system



geometry mapping:

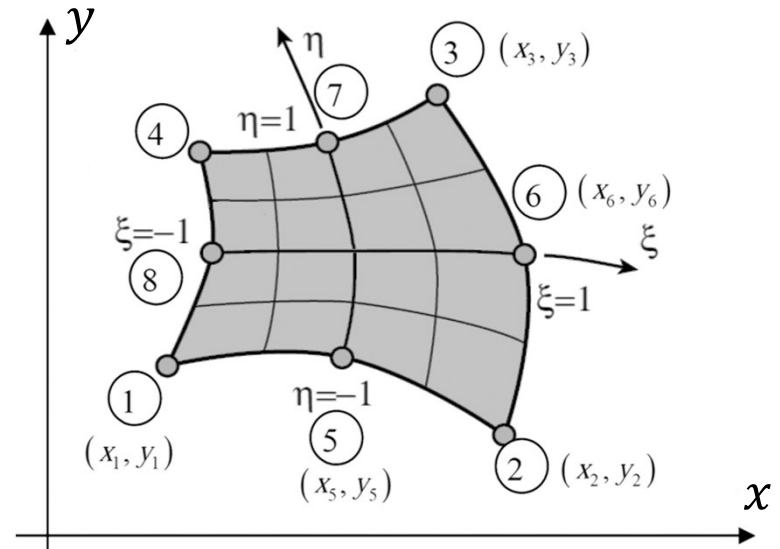
$$(\xi, \eta) \rightarrow (x, y)$$

$$\begin{array}{cccc} (-1, -1) \rightarrow (x_1, y_1) & (1, -1) \rightarrow (x_2, y_2) & (1, 1) \rightarrow (x_3, y_3) & (-1, 1) \rightarrow (x_4, y_4) \\ (0, -1) \rightarrow (x_5, y_5) & (1, 0) \rightarrow (x_6, y_6) & (0, 1) \rightarrow (x_7, y_7) & (-1, 0) \rightarrow (x_8, y_8) \end{array}$$

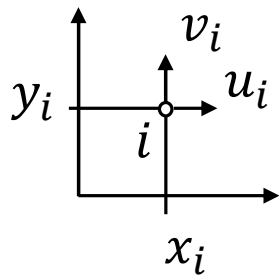
# Isoparametric mapping

vectors of nodal coordinates

$$\{x_i\}_e = \begin{Bmatrix} x_1 \\ x_2 \\ \cdot \\ \cdot \\ \cdot \\ x_8 \end{Bmatrix}_{8 \times 1} ; \{y_i\}_e = \begin{Bmatrix} y_1 \\ y_2 \\ \cdot \\ \cdot \\ \cdot \\ y_8 \end{Bmatrix}_{8 \times 1}$$



local vector of nodal parameters:



$$n = 8 ; n_p = 2 \rightarrow n_e = n \cdot n_p = 16$$

$$\{q\}_e = \begin{Bmatrix} q_1 \\ q_2 \\ \cdot \\ \cdot \\ \cdot \\ \cdot \\ \cdot \\ q_{16} \end{Bmatrix}_e = \begin{Bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \\ \cdot \\ \cdot \\ \cdot \\ u_8 \\ v_8 \end{Bmatrix}_e$$

## Isoparametric mapping

matrix of shape functions:

$$[N(\xi, \eta)]_{2 \times 16} = \begin{bmatrix} N_1(\xi, \eta) & 0 & N_2(\xi, \eta) & 0 & \dots & N_8(\xi, \eta) & 0 \\ 0 & N_1(\xi, \eta) & 0 & N_2(\xi, \eta) & \dots & 0 & N_8(\xi, \eta) \end{bmatrix}$$

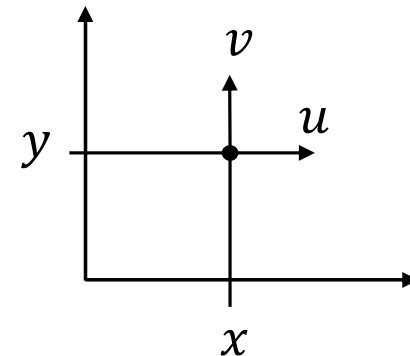
vector of shape functions:

$$[N(\xi, \eta)]_{1 \times 8} = [N_1(\xi, \eta), N_2(\xi, \eta), \dots, N_8(\xi, \eta)]$$

position and displacement of any point:

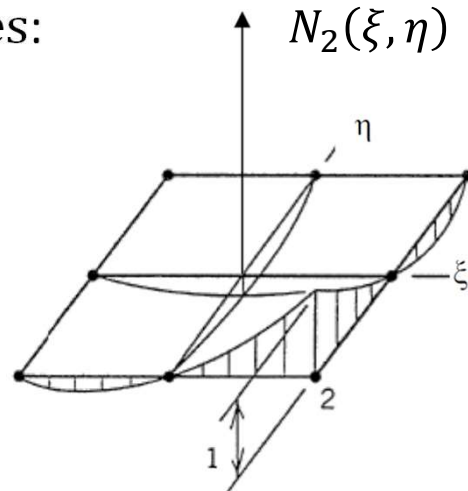
$$x = [N(\xi, \eta)]_{1 \times 8} \{x_i\}_e_{8 \times 1} \quad ; \quad y = [N(\xi, \eta)]_{1 \times 8} \{y_i\}_e_{8 \times 1}$$

$$\{u\}_{2 \times 1} = \begin{Bmatrix} u \\ v \end{Bmatrix} = [N(\xi, \eta)]_{2 \times 16} \{q\}_e_{16 \times 1}$$



# Shape functions of the 8-node quadrilateral finite element

corner nodes:



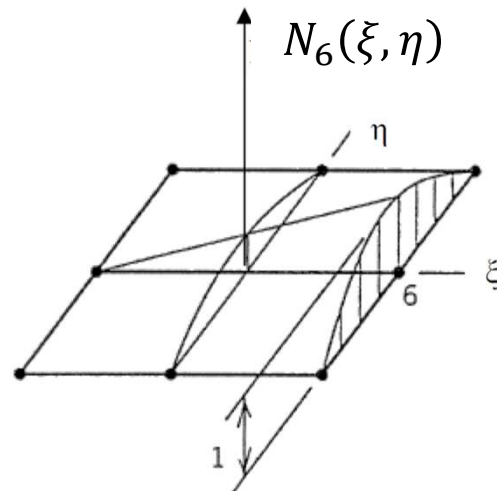
$$N_1(\xi, \eta) = -\frac{1}{4}(1-\xi)(1-\eta)(1+\xi+\eta)$$

$$N_2(\xi, \eta) = -\frac{1}{4}(1+\xi)(1-\eta)(1-\xi+\eta)$$

$$N_3(\xi, \eta) = -\frac{1}{4}(1+\xi)(1+\eta)(1-\xi-\eta)$$

$$N_4(\xi, \eta) = -\frac{1}{4}(1-\xi)(1+\eta)(1+\xi-\eta)$$

midside nodes:



$$N_5(\xi, \eta) = \frac{1}{2}(1-\xi^2)(1-\eta)$$

$$N_6(\xi, \eta) = \frac{1}{2}(1+\xi)(1-\eta^2)$$

$$N_7(\xi, \eta) = \frac{1}{2}(1-\xi^2)(1+\eta)$$

$$N_8(\xi, \eta) = \frac{1}{2}(1-\xi)(1-\eta^2)$$

## Transformation between natural and cartesian coordinates

partial derivatives of any function of coordinates  $(x, y)$  with respect to  $(\xi, \eta)$ :

$$\begin{aligned} \frac{\partial f}{\partial \xi} &= \frac{\partial f}{\partial x} \frac{\partial x}{\partial \xi} + \frac{\partial f}{\partial y} \frac{\partial y}{\partial \xi} \\ \frac{\partial f}{\partial \eta} &= \frac{\partial f}{\partial x} \frac{\partial x}{\partial \eta} + \frac{\partial f}{\partial y} \frac{\partial y}{\partial \eta} \end{aligned} \quad \Rightarrow \quad \begin{pmatrix} \frac{\partial f}{\partial \xi} \\ \frac{\partial f}{\partial \eta} \end{pmatrix} = \begin{bmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial y}{\partial \xi} \\ \frac{\partial x}{\partial \eta} & \frac{\partial y}{\partial \eta} \end{bmatrix} \begin{pmatrix} \frac{\partial f}{\partial x} \\ \frac{\partial f}{\partial y} \end{pmatrix} = \underset{2 \times 2}{[J]} \cdot \begin{pmatrix} \frac{\partial f}{\partial x} \\ \frac{\partial f}{\partial y} \end{pmatrix}$$

$\uparrow$   
*Jacobian matrix*

partial derivatives of any function of coordinates  $(\xi, \eta)$  with respect to  $(x, y)$ :

$$\begin{aligned} \frac{\partial f}{\partial x} &= \frac{\partial f}{\partial \xi} \frac{\partial \xi}{\partial x} + \frac{\partial f}{\partial \eta} \frac{\partial \eta}{\partial x} \\ \frac{\partial f}{\partial y} &= \frac{\partial f}{\partial \xi} \frac{\partial \xi}{\partial y} + \frac{\partial f}{\partial \eta} \frac{\partial \eta}{\partial y} \end{aligned} \quad \Rightarrow \quad \begin{pmatrix} \frac{\partial f}{\partial x} \\ \frac{\partial f}{\partial y} \end{pmatrix} = \begin{bmatrix} \frac{\partial \xi}{\partial x} & \frac{\partial \eta}{\partial x} \\ \frac{\partial \xi}{\partial y} & \frac{\partial \eta}{\partial y} \end{bmatrix} \begin{pmatrix} \frac{\partial f}{\partial \xi} \\ \frac{\partial f}{\partial \eta} \end{pmatrix} = \underset{2 \times 2}{[J]}^{-1} \begin{pmatrix} \frac{\partial f}{\partial \xi} \\ \frac{\partial f}{\partial \eta} \end{pmatrix}$$

$\uparrow$   
*inverse Jacobian matrix*

## Transformation between natural and cartesian coordinates

differential operators:

$$\begin{pmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{pmatrix} = \begin{bmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial y}{\partial \xi} \\ \frac{\partial x}{\partial \eta} & \frac{\partial y}{\partial \eta} \end{bmatrix} \begin{pmatrix} \frac{\partial}{\partial x} \\ \frac{\partial}{\partial y} \end{pmatrix} = \underset{2 \times 2}{[J]} \cdot \begin{pmatrix} \frac{\partial}{\partial x} \\ \frac{\partial}{\partial y} \end{pmatrix} \quad ; \quad \begin{pmatrix} \frac{\partial}{\partial x} \\ \frac{\partial}{\partial y} \end{pmatrix} = \begin{bmatrix} \frac{\partial \xi}{\partial x} & \frac{\partial \eta}{\partial x} \\ \frac{\partial \xi}{\partial y} & \frac{\partial \eta}{\partial y} \end{bmatrix} \begin{pmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{pmatrix} = \underset{2 \times 2}{[J]^{-1}} \cdot \begin{pmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{pmatrix}$$

*Jacobian matrix*

*inverse Jacobian matrix*

differential operators:

$$\begin{pmatrix} \frac{\partial}{\partial x} \\ \frac{\partial}{\partial y} \end{pmatrix} = \underset{2 \times 2}{[J]^{-1}} \begin{pmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{pmatrix} \quad ; \quad \underset{2 \times 2}{[J]} \begin{pmatrix} \frac{\partial}{\partial x} \\ \frac{\partial}{\partial y} \end{pmatrix} = \underset{2 \times 2}{[J]} \cdot \underset{2 \times 2}{[J]^{-1}} \begin{pmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{pmatrix} = \underset{2 \times 2}{[I]} \cdot \begin{pmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{pmatrix} = \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix} \begin{pmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{pmatrix} = \begin{pmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{pmatrix}$$

*unit matrix*

# Transformation between natural and cartesian coordinates

inverse Jakobian matrix:

$$\begin{aligned}
 [J]_{2 \times 2}^{-1} &= \frac{1}{\det[J]} ([J]^C)_{2 \times 2}^T = \frac{1}{\det[J]} \left( \begin{bmatrix} \frac{\partial y}{\partial \eta} & -\frac{\partial x}{\partial \eta} \\ -\frac{\partial y}{\partial \xi} & \frac{\partial x}{\partial \xi} \end{bmatrix} \right)^T = \frac{1}{\det[J]} \begin{bmatrix} \frac{\partial y}{\partial \eta} & -\frac{\partial y}{\partial \xi} \\ -\frac{\partial x}{\partial \eta} & \frac{\partial x}{\partial \xi} \end{bmatrix} = \\
 &= \begin{bmatrix} \frac{1}{\det[J]} \frac{\partial y}{\partial \eta} & -\frac{1}{\det[J]} \frac{\partial y}{\partial \xi} \\ -\frac{1}{\det[J]} \frac{\partial x}{\partial \eta} & \frac{1}{\det[J]} \frac{\partial x}{\partial \xi} \end{bmatrix}
 \end{aligned}$$

↑  
*cofactors matrix*

$$\begin{aligned}
 [J]_{2 \times 2}^{-1} &= \begin{bmatrix} \frac{\partial \xi}{\partial x} & \frac{\partial \eta}{\partial x} \\ \frac{\partial \xi}{\partial y} & \frac{\partial \eta}{\partial y} \end{bmatrix} \rightarrow \begin{array}{l}
 \frac{\partial \xi}{\partial x} = \frac{1}{\det[J]} \frac{\partial y}{\partial \eta} = \frac{1}{\det[J]} \frac{\partial (|N(\xi, \eta)| \{y_i\}_e)}{\partial \eta} = \frac{1}{\det[J]} \frac{\partial |N(\xi, \eta)|}{\partial \eta} \{y_i\}_e \\
 \frac{\partial \eta}{\partial x} = -\frac{1}{\det[J]} \frac{\partial y}{\partial \xi} = -\frac{1}{\det[J]} \frac{\partial (|N(\xi, \eta)| \{y_i\}_e)}{\partial \xi} = -\frac{1}{\det[J]} \frac{\partial |N(\xi, \eta)|}{\partial \xi} \{y_i\}_e \\
 \frac{\partial \xi}{\partial y} = -\frac{1}{\det[J]} \frac{\partial x}{\partial \eta} = -\frac{1}{\det[J]} \frac{\partial (|N(\xi, \eta)| \{x_i\}_e)}{\partial \eta} = -\frac{1}{\det[J]} \frac{\partial |N(\xi, \eta)|}{\partial \eta} \{x_i\}_e \\
 \frac{\partial \eta}{\partial y} = \frac{1}{\det[J]} \frac{\partial x}{\partial \xi} = \frac{1}{\det[J]} \frac{\partial (|N(\xi, \eta)| \{x_i\}_e)}{\partial \xi} = \frac{1}{\det[J]} \frac{\partial |N(\xi, \eta)|}{\partial \xi} \{x_i\}_e
 \end{array}
 \end{aligned}$$



## Transformation between natural and cartesian coordinates

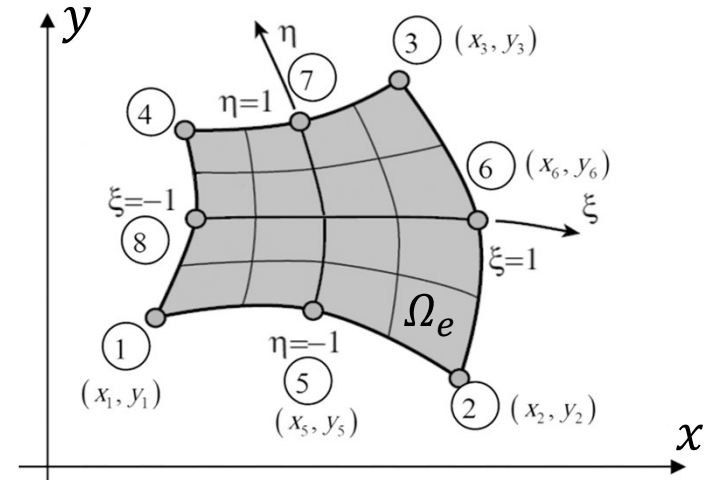
determinant of the Jakobian matrix:

$$\det[J] = \det \begin{bmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial y}{\partial \xi} \\ \frac{\partial x}{\partial \eta} & \frac{\partial y}{\partial \eta} \end{bmatrix} = \frac{\partial x}{\partial \xi} \frac{\partial y}{\partial \eta} - \frac{\partial y}{\partial \xi} \frac{\partial x}{\partial \eta} =$$

$$= \frac{\partial([N(\xi,\eta)]\{x_i\}_e)}{\partial \xi} \frac{\partial([N(\xi,\eta)]\{y_i\}_e)}{\partial \eta} - \frac{\partial([N(\xi,\eta)]\{y_i\}_e)}{\partial \xi} \frac{\partial([N(\xi,\eta)]\{x_i\}_e)}{\partial \eta} =$$

$$= \left( \frac{\partial[N(\xi,\eta)]}{\partial \xi} \right)_{1 \times 8} \left( \{x_i\}_e \right)_{8 \times 1} \left( \frac{\partial[N(\xi,\eta)]}{\partial \eta} \right)_{1 \times 8} \left( \{y_i\}_e \right)_{8 \times 1} - \left( \frac{\partial[N(\xi,\eta)]}{\partial \xi} \right)_{1 \times 8} \left( \{y_i\}_e \right)_{8 \times 1} \left( \frac{\partial[N(\xi,\eta)]}{\partial \eta} \right)_{1 \times 8} \left( \{x_i\}_e \right)_{8 \times 1}$$

*(known at any point of the domain  $\Omega_e$ )*



## Gradient matrix calculation

differential operators in the coordinate system  $(x, y)$ :

$$\begin{Bmatrix} \frac{\partial}{\partial x} \\ \frac{\partial}{\partial y} \end{Bmatrix} = [J]_{2 \times 2}^{-1} \begin{Bmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{Bmatrix} = \begin{bmatrix} \frac{1}{\det[J]} \frac{\partial y}{\partial \eta} & -\frac{1}{\det[J]} \frac{\partial y}{\partial \xi} \\ -\frac{1}{\det[J]} \frac{\partial x}{\partial \eta} & \frac{1}{\det[J]} \frac{\partial x}{\partial \xi} \end{bmatrix} \begin{Bmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{Bmatrix} \rightarrow$$

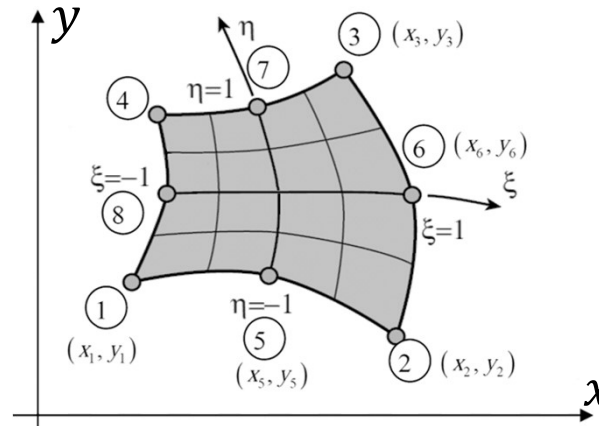
$$\frac{\partial}{\partial x} = \frac{1}{\det[J]} \frac{\partial y}{\partial \eta} \frac{\partial}{\partial \xi} - \frac{1}{\det[J]} \frac{\partial y}{\partial \xi} \frac{\partial}{\partial \eta} \quad ; \quad \frac{\partial}{\partial y} = -\frac{1}{\det[J]} \frac{\partial x}{\partial \eta} \frac{\partial}{\partial \xi} + \frac{1}{\det[J]} \frac{\partial x}{\partial \xi} \frac{\partial}{\partial \eta}$$

gradient matrix for plane stress or plane strain conditions:

$$[R]_{3 \times 2} = \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} \end{bmatrix} = \begin{bmatrix} \left( \frac{1}{\det[J]} \frac{\partial y}{\partial \eta} \frac{\partial}{\partial \xi} - \frac{1}{\det[J]} \frac{\partial y}{\partial \xi} \frac{\partial}{\partial \eta} \right) & 0 \\ 0 & \left( \frac{1}{\det[J]} \frac{\partial x}{\partial \xi} \frac{\partial}{\partial \eta} - \frac{1}{\det[J]} \frac{\partial x}{\partial \eta} \frac{\partial}{\partial \xi} \right) \\ \left( \frac{1}{\det[J]} \frac{\partial x}{\partial \xi} \frac{\partial}{\partial \eta} - \frac{1}{\det[J]} \frac{\partial x}{\partial \eta} \frac{\partial}{\partial \xi} \right) & \left( \frac{1}{\det[J]} \frac{\partial y}{\partial \eta} \frac{\partial}{\partial \xi} - \frac{1}{\det[J]} \frac{\partial y}{\partial \xi} \frac{\partial}{\partial \eta} \right) \end{bmatrix} = [R(\xi, \eta)]_{3 \times 2}$$

## Gradient matrix for plane stress and plane strain conditions

$$[R(\xi, \eta)]_{3 \times 2} = \begin{bmatrix} \frac{\partial}{\partial x}(\xi, \eta) & 0 \\ 0 & \frac{\partial}{\partial y}(\xi, \eta) \\ \frac{\partial}{\partial y}(\xi, \eta) & \frac{\partial}{\partial x}(\xi, \eta) \end{bmatrix}$$



$$\{x_i\}_e = \begin{Bmatrix} x_1 \\ x_2 \\ \cdot \\ \cdot \\ \cdot \\ x_8 \end{Bmatrix}_{8 \times 1}$$

$$\{y_i\}_e = \begin{Bmatrix} y_1 \\ y_2 \\ \cdot \\ \cdot \\ \cdot \\ y_8 \end{Bmatrix}_{8 \times 1}$$

$$\frac{\partial}{\partial x}(\xi, \eta) = \frac{\left( \frac{\partial[N(\xi, \eta)]}{\partial \eta} \{y_i\}_e \right) \frac{\partial}{\partial \xi} - \left( \frac{\partial[N(\xi, \eta)]}{\partial \xi} \{y_i\}_e \right) \frac{\partial}{\partial \eta}}{\left( \frac{\partial[N(\xi, \eta)]}{\partial \xi} \{x_i\}_e \right) \left( \frac{\partial[N(\xi, \eta)]}{\partial \eta} \{y_i\}_e \right) - \left( \frac{\partial[N(\xi, \eta)]}{\partial \xi} \{y_i\}_e \right) \left( \frac{\partial[N(\xi, \eta)]}{\partial \eta} \{x_i\}_e \right)}$$

$$\frac{\partial}{\partial y}(\xi, \eta) = \frac{\left( \frac{\partial[N(\xi, \eta)]}{\partial \xi} \{x_i\}_e \right) \frac{\partial}{\partial \eta} - \left( \frac{\partial[N(\xi, \eta)]}{\partial \eta} \{x_i\}_e \right) \frac{\partial}{\partial \xi}}{\left( \frac{\partial[N(\xi, \eta)]}{\partial \xi} \{x_i\}_e \right) \left( \frac{\partial[N(\xi, \eta)]}{\partial \eta} \{y_i\}_e \right) - \left( \frac{\partial[N(\xi, \eta)]}{\partial \xi} \{y_i\}_e \right) \left( \frac{\partial[N(\xi, \eta)]}{\partial \eta} \{x_i\}_e \right)}$$

## Strain energy in the 8-node QUAD element

strain vector for plane stress or plane strain conditions:

$$\{\varepsilon\} = [R(\xi, \eta)]\{u\} = [R(\xi, \eta)][N(\xi, \eta)]\{q\}_e = [B(\xi, \eta)]\{q\}_e$$

$\begin{matrix} 3 \times 1 & 3 \times 2 & 2 \times 1 & 3 \times 2 & 2 \times 16 & 16 \times 1 & 3 \times 16 & 16 \times 1 \end{matrix}$

elastic strain energy in a finite element:

$$U_e = \frac{1}{2} \int_{\Omega_e} [\varepsilon] \{\sigma\} d\Omega_e = \frac{1}{2} [q]_e (t_e \int_{A_e} [B(\xi, \eta)]^T [D] [B(\xi, \eta)] dx dy) \{q\}_e =$$

$\begin{matrix} 1 \times 3 & 1 \times 16 & 1 \times 3 & 3 \times 1 & 1 \times 16 & A_e & 16 \times 3 & 3 \times 3 & 3 \times 16 & 16 \times 1 \end{matrix}$

$$= \frac{1}{2} [q]_e [k]_e \{q\}_e$$

$$[\varepsilon] = [q]_e [B(\xi, \eta)]^T$$

$\begin{matrix} 1 \times 3 & 1 \times 16 & 16 \times 3 \end{matrix}$

$$\{\sigma\} = [D] \{\varepsilon\}$$

$\begin{matrix} 3 \times 1 & 3 \times 3 & 3 \times 1 \end{matrix}$

$$\{q\}_e = [B(\xi, \eta)]\{q\}_e$$

$\begin{matrix} 3 \times 1 & 3 \times 16 & 16 \times 1 \end{matrix}$

$$\left( \int_{A_e} f(x, y) dx dy = \int_{-1}^1 \int_{-1}^1 f(\xi, \eta) \det[J] d\xi d\eta \right)$$

local stiffness matrix:

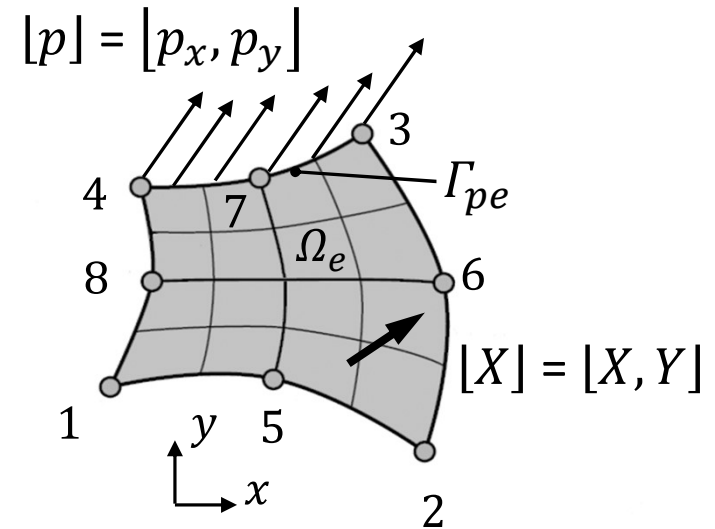
$$[k]_e = t_e \int_{-1}^1 \int_{-1}^1 [B(\xi, \eta)]^T [D] [B(\xi, \eta)] \det[J] d\xi d\eta \quad (\text{calculated numerically})$$

$\begin{matrix} 16 \times 16 & 16 \times 3 & 3 \times 3 & 3 \times 16 \end{matrix}$

# Potential energy of loading and equivalent load vector

potential energy of loading in a finite element:

$$\begin{aligned}
 W_e &= \int_{\Omega_e} [X] \{u\} d\Omega_e + \int_{\Gamma_{pe}} [p] \{u\} d\Gamma_{pe} = \\
 &\quad \begin{matrix} \Omega_e & 1 \times 2 & 2 \times 1 \\ \Gamma_{pe} & 1 \times 2 & 2 \times 1 \end{matrix} \\
 &\quad \{u\} = [N] \{q\}_e \\
 &\quad \begin{matrix} 2 \times 1 & 2 \times 16 & 16 \times 1 \end{matrix} \\
 &= \left( \int_{\Omega_e} [X][N] d\Omega_e + \int_{\Gamma_{pe}} [p][N] d\Gamma_{pe} \right) \{q\}_e = \\
 &\quad \begin{matrix} \Omega_e & 1 \times 2 & 2 \times 16 \\ \Gamma_{pe} & 1 \times 2 & 2 \times 16 \end{matrix} \quad \begin{matrix} 16 \times 1 \end{matrix} \\
 &= ([F^X]_e + [F^p]_e) \{q\}_e = [F]_e \{q\}_e \\
 &\quad \begin{matrix} 1 \times 16 & 1 \times 16 & 16 \times 1 & 1 \times 16 & 16 \times 1 \end{matrix}
 \end{aligned}$$



$$[F^X]_e = t_e \int_{-1}^1 \int_{-1}^1 [X(\xi, \eta)] [N(\xi, \eta)] \det[J] d\xi d\eta$$

$\begin{matrix} 1 \times 16 & 1 \times 2 & 2 \times 16 \end{matrix}$

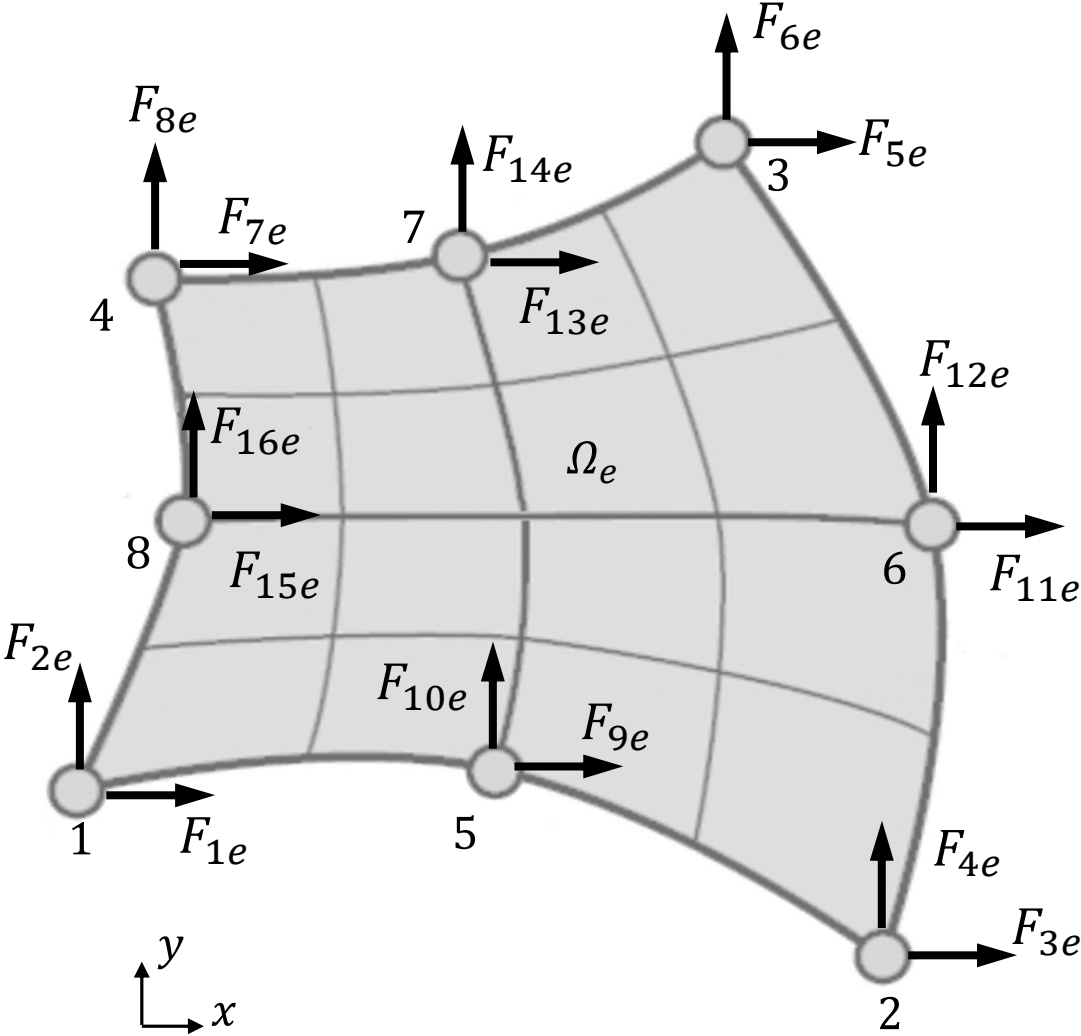
$$[F^p]_e = \int_{\Gamma_{pe}} [p][N] d\Gamma_{pe}$$

$\begin{matrix} 1 \times 16 & 1 \times 2 & 2 \times 16 \end{matrix}$

# Equivalent load vector in the 8-node quadrilateral element

$$[F]_e$$

$$16 \times 1$$

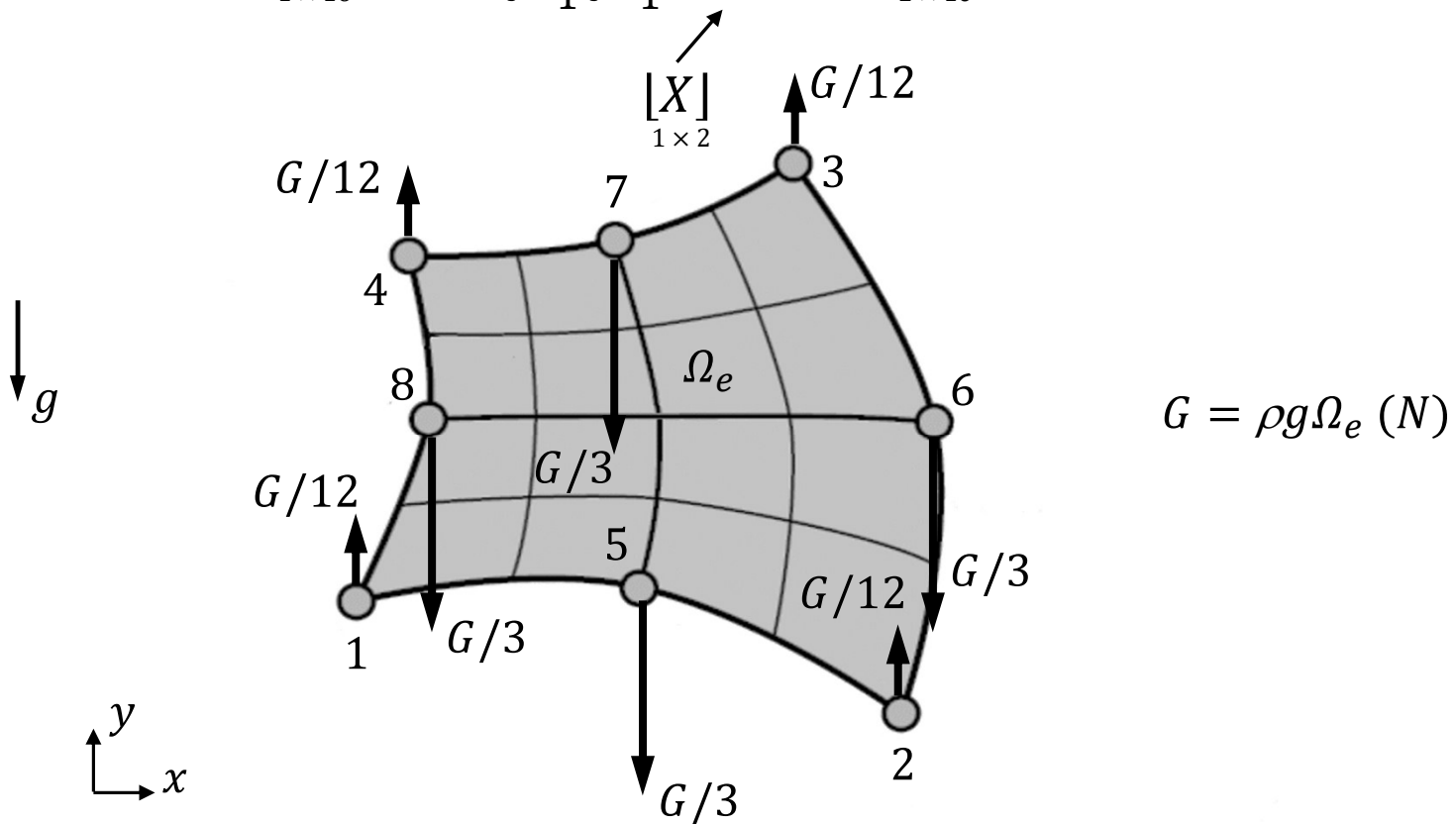


## Example. Equivalent load vector due to mass forces (gravity load)

gravity load:

$$[F^X]_e = t_e \int_{-1}^1 \int_{-1}^1 [0, -\rho g] [N(\xi, \eta)] \det[J] d\xi d\eta$$

$1 \times 16$                        $1 \times 2$                        $2 \times 16$



$$G = \rho g \Omega_e \text{ (N)}$$

$$[F]_e = \left[ 0, \frac{G}{12}, 0, \frac{G}{12}, 0, \frac{G}{12}, 0, \frac{G}{12}, 0, -\frac{G}{3}, 0, -\frac{G}{3}, 0, -\frac{G}{3}, 0, -\frac{G}{3} \right]_e$$

## Equivalent load vector due to surface load

equivalent load vector due to surface load:

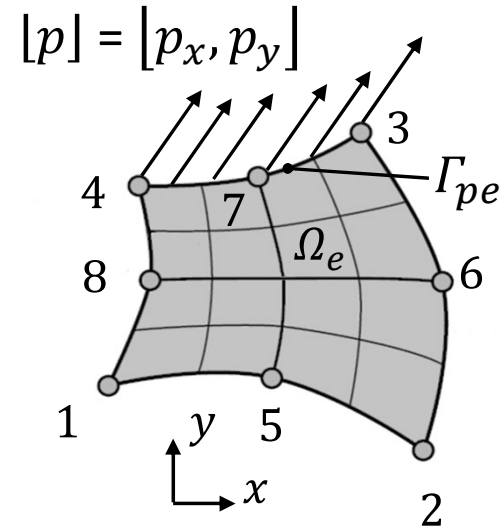
$$[F^p]_e = \int_{\Gamma_{pe}} [p][N] d\Gamma_{pe} = t_e \int_0^l [p][N] ds$$

$1 \times 16$                        $\Gamma_{pe}$   $1 \times 2$   $2 \times 16$

$$= t_e \int_0^l [p][N] ds = t_e \int_{-1}^1 [p][N] \frac{ds}{d\xi} d\xi$$

$1 \times 2$   $2 \times 16$                        $1 \times 2$   $2 \times 16$

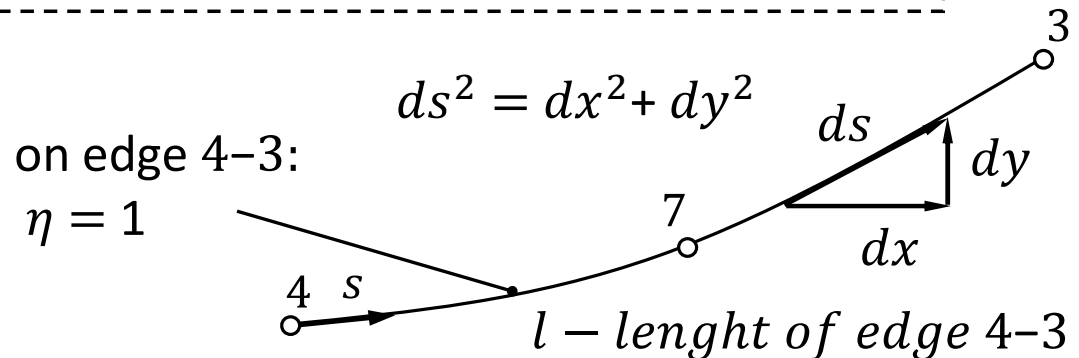
$$\frac{ds^2}{d\xi^2} = \frac{dx^2}{d\xi^2} + \frac{dy^2}{d\xi^2} \rightarrow \frac{ds}{d\xi} = \sqrt{\left(\frac{dx}{d\xi}\right)^2 + \left(\frac{dy}{d\xi}\right)^2}$$



$$[F^p]_e = t_e \int_{-1}^1 [p_x, p_y][N] \sqrt{\left(\frac{\partial[N(\xi,1)]}{\partial\xi} \{x_i\}_e\right)^2 + \left(\frac{\partial[N(\xi,1)]}{\partial\xi} \{y_i\}_e\right)^2} d\xi$$

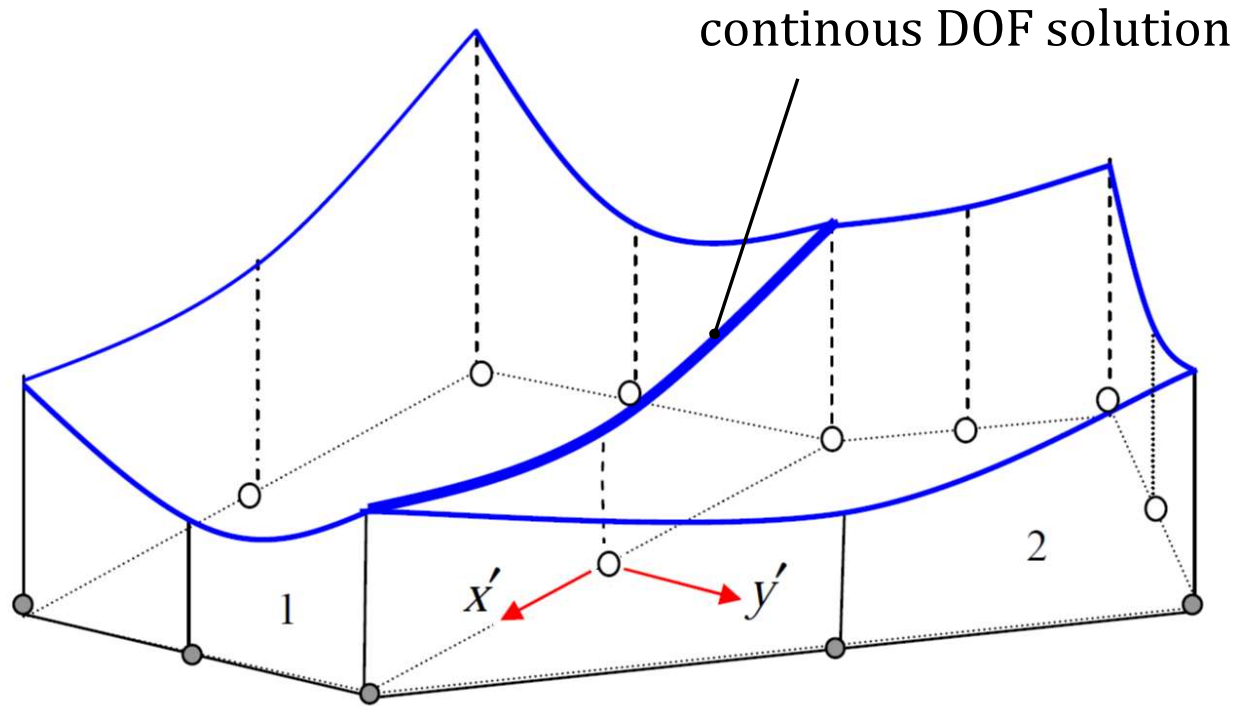
$1 \times 16$                        $2 \times 16$                        $1 \times 8$                        $8 \times 1$                        $1 \times 8$                        $8 \times 1$

(calculated numerically)





# Results on the edge between two 8-node FEs

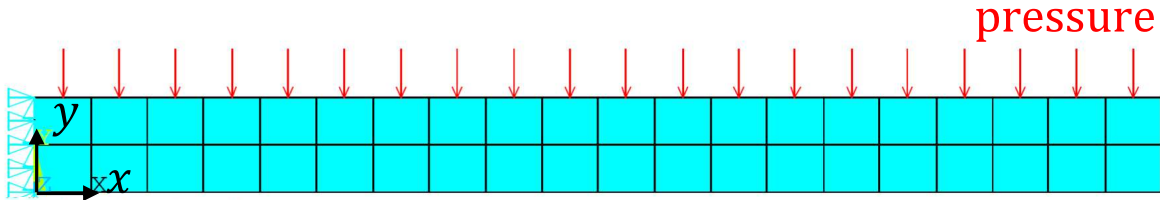


$$\begin{aligned}
 \frac{\partial u}{\partial x'} \Big|_1 &= \frac{\partial u}{\partial x'} \Big|_2 & \Rightarrow & \quad (\epsilon_{x'})_1 = (\epsilon_{x'})_2 & \Rightarrow & \quad (\sigma_{ij})_1 \neq (\sigma_{ij})_2 \\
 \frac{\partial u}{\partial y'} \Big|_1 &\neq \frac{\partial u}{\partial y'} \Big|_2 & \Rightarrow & \quad (\epsilon_{y'})_1 \neq (\epsilon_{y'})_2 & & 
 \end{aligned}$$

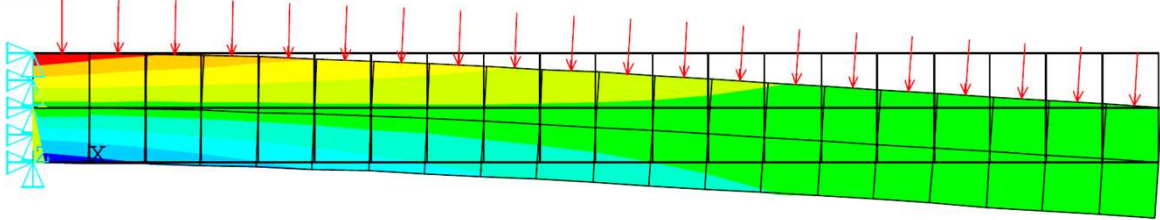
not continuous element solution

# Example. 2D model of a centilever beam (8-node FEs)

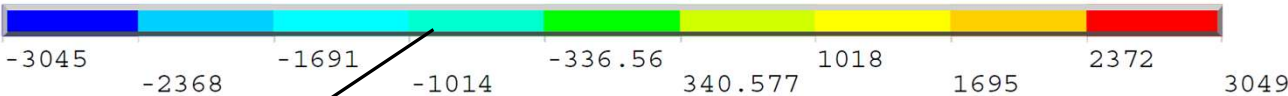
fixed support



element solution

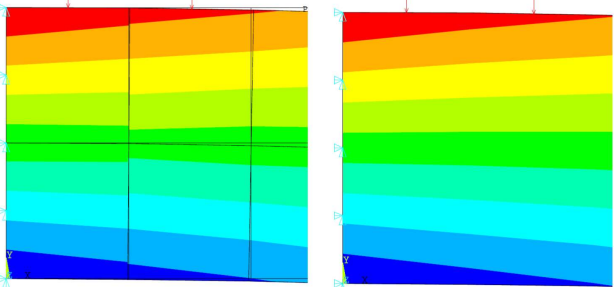


$\sigma_x$



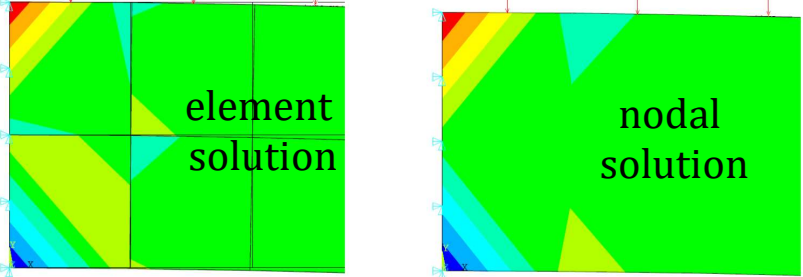
stress  $\sigma_x$

stress  $\sigma_y$



element solution

nodal solution



element solution

nodal solution

